

For a flexural member to be adequate in deflection the summation of the actual deflection due to bending  $\delta_m$  and that due to shear  $\delta_v$  must not be greater than the permissible value  $\delta_p$ :

$$\delta_m + \delta_v \leq \delta_p$$

The actual bending deflection  $\delta_m$  is calculated using the formula relevant to the applied loading:

For a UDL:

$$\delta_m = \frac{5}{384} \frac{WL^3}{EI}$$

For a central point load:

$$\delta_m = \frac{1}{48} \frac{WL^3}{EI}$$

The  $E$  value to be adopted is  $E_{\min}$  for isolated members and  $E_{\text{mean}}$  for load sharing members.

The actual deflection  $\delta_v$  produced by shear on rectangular (and square) cross-section members is calculated using the following expression:

$$\delta_v = \frac{SM}{AG}$$

where

$M$  mid-span bending moment

$S$  member shape factor, which is 1.2 for rectangular sections

$A$  section area

$G$  modulus of rigidity or shear modulus, taken as  $E/16$

$E$  appropriate  $E$  value:  $E_{\min}$  for isolated members,  $E_{\text{mean}}$  for load sharing members

The expression can be rewritten to include the values for  $S$  and  $G$  as follows:

$$\delta_v = \frac{SM}{AG} = \frac{1.2M}{AE/16} = \frac{19.2M}{AE}$$

The total actual deflection  $\delta_a$  resulting from bending and shear will therefore be the summation of the relevant formulae:

$$\delta_a = \delta_m + \delta_v$$

For a UDL:

$$\delta_a = \frac{5}{384} \frac{WL^3}{EI} + \frac{19.2M}{AE}$$

For a point load:

$$\delta_a = \frac{1}{48} \frac{WL^3}{EI} + \frac{19.2M}{AE}$$

### 2.12.3 Shear

The critical position for shear on a normally loaded flexural member is at the support where the maximum reaction occurs. The shear stress occurring at that position is calculated and compared with the permissible value.

For rectangular timber flexural members the maximum applied shear stress parallel to the grain,  $r_a$ , occurs at the NA and is calculated from the following expression:

$$r_a = \frac{3 F_v}{2 A}$$

where  $F_v$  is the maximum vertical shear (usually maximum reaction) and  $A$  is the cross-sectional area ( $bh$ ).

The applied shear stress must be less than the permissible shear stress parallel to the grain,  $r_{adm}$ . This is obtained by multiplying the grade shear stress parallel to the grain,  $r_g$  (Table 2.2), by the  $K_3$  load duration and  $K_8$  load sharing factors as appropriate. Thus

$$\begin{aligned} r_a &\leq r_{adm} \\ r_a &\leq r_g K_3 K_8 \end{aligned}$$

Notches occurring at the ends of flexural members affect their shear capacity. To allow for this, the permissible shear stress parallel to the grain is further reduced by a modification factor  $K_5$ . Thus

$$r_a = \frac{3 F_v}{2 A} \leq r_{adm} K_5$$

The factor  $K_5$  is obtained in one of two ways depending on whether the notch occurs in the top (Figure 2.1) or the bottom (Figure 2.2) of the member:

$$\text{Top edge notches: } K_5 = \begin{cases} \frac{h(h_e - a) + ah_e}{h_e^2} & \text{when } a \leq h_e \\ 1.0 & \text{when } a > h_e \end{cases}$$

$$\text{Bottom edge notches: } K_5 = \frac{h_e}{h}$$

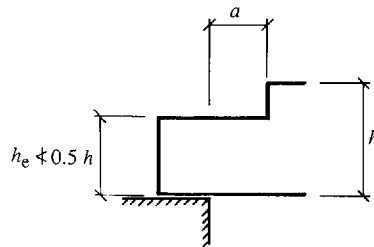


Figure 2.1 Top edge notch

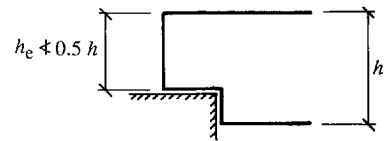


Figure 2.2 Bottom edge notch